

March 11, 2020

Mr. Scott Drumbore, P.E.
Division Manager – Engineering & Environmental Services Division
H&K Group, Inc.
2052 Lucon Road
P.O. Box 196
Skippack, PA 19474

MCMAHON ASSOCIATES, INC. 1515 Market Street, Suite 1360 Philadelphia, PA 19102 p 215-433-1660 I 6215-433-1661

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FOUNDER

Joseph W. McMahon, P.E.

RE: Trip Generation Letter
Center Valley Materials
Springfield Township, Bucks County, PA
McMahon Project No. 820220.11

Dear Scott:

Per your request, McMahon Associates, Inc. (McMahon) has completed a Trip Generation evaluation and letter report for submission to assess the existing operations, and proposed development, located along Springfield Street, east of N. West End Boulevard (S.R. 0309) in Springfield Township, Bucks County, PA.

The existing materials and trucking operations, known as Coopersburg Materials, consists of an occupied asphalt mixing plant on the north side of Springfield Street. The proposed development and operations, being referred to as Center Valley Materials, will consist of a G-7 Quarry use with access proposed via a single driveway on the south side of Springfield Street, directly opposite the existing Coopersburg Materials asphalt plant. The proposed quarry will provide stone to the existing asphalt plant, as well as other locations within the area. The stone and aggregate currently being processed at the asphalt plant is delivered from an existing quarry, Naceville Materials to the south in West Rockhill Township. Trucks currently traveling to and from the asphalt plant transporting stone travel along S.R. 0309 and Springfield Street to deliver the stone and then return to the quarry. The proposed Center Valley Materials quarry will become the new source for the stone and aggregate. Therefore, these existing truck trips will no longer utilize the existing roadway network to access Coopersburg Materials.

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Trip Generation for Coopersburg Materials and Center Valley Materials

Peak hour and daily trip generation of the existing site, as well as the proposed modification of the site, was based on information provided by H&K Group Inc., herein H&K. Estimates of traffic volumes generated by any development and land use are typically prepared based on trip generation data compiled from numerous studies contained in the Institute of Transportation Engineers (ITE) publication, Trip Generation, 10th Edition (2017). However, due to the land use of the asphalt batch plant and quarry, McMahon was able to utilize information based on actual trip data and activity provided in H&K's log data and correspondence for the existing asphalt batch plant and future proposed G-7 Quarry use. H&K's peak season log data is attached.

94 (58) (54)

Based on email correspondence, H&K provided McMahon with the following information:

- Log data of all loads from the Naceville quarry to Coopersburg Materials from January 2019 -December 2019.
- Log data of all loads from the H&K Materials quarry, in Hilltown Township from January 2019 - December 2019.
- Each load represents 1 enter trip and 1 exit trip.
- The proposed site is expected to produce a similar number of trips as the existing H&K Materials quarry based upon an annual production of 500,000 tons per year.
- Once the proposed Center Valley Materials site is complete, trips from the Naceville quarry to Coopersburg Materials will cease.
- Existing site traffic is primarily truck traffic.
- Approximately 10 people will be employed at the proposed site.
- Employees at the proposed site will typically arrive at 6:00 AM.
- Approximately 15 miscellaneous trips will occur daily at the proposed site.
- Busy season for the existing site occurs between the months of April and November with typical peaks between August and September.
- The existing weekday morning peak hour occurs from 7:00-9:00 AM.
- The existing weekday afternoon peak hour occurs from 4:00-6:00 PM.

Based on the data provided, and phone calls between our office and H&K, McMahon developed the following assumptions to calculate Center Valley Materials site trips:

- Assumed the months of August September as the typical busiest season for this site and industry-wide. Used log information from August 2019 - September 2019 to calculate Center Valley Materials site trips.
- Used 7:00-9:00 AM and 4:00-6:00 PM as the peak hours. (matches commuter peak periods)

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- Calculated the average amount of trips for the weekday morning, weekday afternoon, and daily from Monday-Friday in August 2019 – September 2019.
- Existing trip generation for the H&K Materials quarry is expected to be similar due to anticipated production rates for the Center Valley Materials quarry.
- Existing trip generation from the Naceville quarry to Coopersburg Materials will be eliminated in proposed conditions.

The trip generation characteristics for the existing site and the future proposed modification of the site were calculated based on information provided by H&K. Calculations for the trip generation are attached.

Table 1 provides a summary of the anticipated trips for the existing site utilizing information from H&K.

Land Use	Dalla	Wee	kday Mor	ning	Weel	Weekday Afternoon		
Land Ose	Daily	In	Out	Total	În	Out	Total	
Proposed Quarry	319(1)	37	37	74	0	10	10	
Existing Asphalt Plant	116	7	7	14	3	3	6	
Total "New" Trips	203(1)	30	30	60	-3	7	4	

Table 1 - Vehicular Trip Generation Characteristics

As shown in Table 1, based on log sheet data from the existing Naceville quarry, Center Valley Materials is anticipated to generate approximately 319 daily trips, with approximately 74 total trips during the weekday morning peak hour, and approximately 10 total trips during the weekday afternoon peak hour. Based on the log data, vehicles providing quarry supplied material to/from the existing asphalt plant is calculated to be approximately 116 total daily trips, approximately 14 total trips during the weekday morning peak hour, and approximately 6 total trips during the weekday afternoon peak hour.

Since the existing trips from the Naceville quarry to Coopersburg Materials will be replaced by trips immediately crossing Springfield Street between the Center Valley Materials and Coopersburg Materials sites in the future, the net impact on the roadway network will be approximately 203 total daily trips, approximately 60 total trips during the weekday morning peak hour, and approximately 4 total trips during the weekday afternoon peak hour.

⁽¹⁾ Daily trips include truck trips from the log data plus trips for the 10 employees and 15 miscellaneous trips.

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Based on our evaluation of trips associated with the proposed quarry use, it is our professional opinion that the traffic generated by the G-7 Quarry use will not have a significant impact on the operations of Springfield Street and its intersection with Route 309. Our opinion will be further confirmed with the completion of the Traffic Study.

We trust that this Trip Generation evaluation and letter responds to your requests and satisfactorily addresses the traffic-related questions at this time that are related to the proposed G-7 Quarry use and related operations in the future. If you have any questions, or require further clarification, please feel free to contact me.

Sincerely,

Mark A. Roth, P.E.

General Manager – Philadelphia Office

MAR/das

Attachments

cc: Derek A. Shew, McMahon Associates, Inc.

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Outgoing (Naceville Materials to Coopersburg Materials)

August	Daily	7-9 AM	4-6 PM
7th	31	3	5
14th	71	8	0
21st	62	8	4
28th	56	5	2
September	Daily	7-9 AM	4-6 PM
4th	58	5	2
11th	48	5	6
18th	56	8	2
25th	85	13	2
Average (Exiting Trips)	58	7	3
Assumed (Entering Trips)	58	7	3
Assumed Total Passenger Vehicle			
Trips (Exiting + Entering)	0	0	0
Assumed Existing Total Trips (Exiting + Entering)	116	14	6

Outgoing (Center Valley Materials)

August	Daily	7-9 AM	4-6 PM
7th	141	39	0
14th	126	35	0
21st	102	22	0
28th	96	21	0
September	Daily	7-9 AM	4-6 PM
4th	178	49	0
11th	188	51	0
18th	178	40	0
25th	166	38	0
Average (Exiting Trips)	147	37	0
Assumed (Entering Trips)	147	37	0
Assumed Total Passenger Vehicle			
Trips (Exiting + Entering)	25	0	10
Assumed Existing Total Trips (Exiting + Entering)	319	74	10

Proposed Net Trips

August	Daily	7-9 AM	4-6 PM
7th	110	36	-5
14th	55	27	0
21st	40	14	-4
28th	40	16	-2
September	Daily	7-9 AM	4-6 PM
4th	120	44	-2
11th	140	46	-6
18th	122	32	-2
25th	81	25	-2
Average (Exiting Trips)	89	30	-3
Assumed (Entering Trips)	89	30	-3
Assumed Total Passenger Vehicle			
Trips (Exiting + Entering)	25	0	10
Assumed Existing Total Trips (Exiting + Entering)	203	60	4

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328898			COOPERSBURG MATERIALS	24.72
328899	4/3/2019	7:06:00 AM	COOPERSBURG MATERIALS	21.75
328915	4/3/2019	7:49:00 AM	COOPERSBURG MATERIALS	23.81
328919	4/3/2019	7:59:00 AM	COOPERSBURG MATERIALS	21.92
328921	4/3/2019	8:03:00 AM	COOPERSBURG MATERIALS	24.26
328924	4/3/2019	8:11:00 AM	COOPERSBURG MATERIALS	21.42
328934	4/3/2019	8:44:00 AM	COOPERSBURG MATERIALS	24.13
328942	4/3/2019	8:59:00 AM	COOPERSBURG MATERIALS	22.41
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329105	4/3/2019	4:19:00 PM	COOPERSBURG MATERIALS	23.94
329107	4/3/2019	4:23:00 PM	COOPERSBURG MATERIALS	23.25
329109	4/3/2019	4:37:00 PM	COOPERSBURG MATERIALS	22.58
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330349	4/10/2019	7:14:00 AM	COOPERSBURG MATERIALS	22.24
330353	4/10/2019		COOPERSBURG MATERIALS	23.89
330354	4/10/2019		COOPERSBURG MATERIALS	21.77
330356	4/10/2019		COOPERSBURG MATERIALS	24.63
330358	4/10/2019		COOPERSBURG MATERIALS	20.82
330371	4/10/2019		COOPERSBURG MATERIALS	22.33
330375	4/10/2019	8:25:00 AM	COOPERSBURG MATERIALS	24.13
330377	4/10/2019	8:35:00 AM	COOPERSBURG MATERIALS	21.89
330379	4/10/2019	8:43:00 AM	COOPERSBURG MATERIALS	23.99
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331605	4/17/2019		COOPERSBURG MATERIALS	24.78
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331630	4/17/2019	8:51:00 AM	COOPERSBURG MATERIALS	22.17
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332763	4/24/2019	7:13:00 AM C	COOPERSBURG MATERIALS	22.42
332770	4/24/2019	7:46:00 AM C	COOPERSBURG MATERIALS	24.13
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332776	4/24/2019		COOPERSBURG MATERIALS	22.25
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327264	4/3/2019	7:43:00 AM	HWSA PFC Treatment Group 2	22.2
443237	4/3/2019	7:47:00 AM	COSTARS 5610-15 PERKASIE BORO	3.11
327265	4/3/2019	7:52:00 AM	HWSA PFC Treatment Group 2	2:
443238	4/3/2019	7:53:00 AM	B & L 2019 SEASONAL	15.78
443239	4/3/2019	8:02:00 AM	DOYLESTOWN HOSPITAL PAVILLION III	22.1
443240	4/3/2019	8:04:00 AM	H.D.PAVING	21.5
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443242	4/3/2019		DOYLESTOWN HOSPITAL PAVILLION III	21.3
443243	4/3/2019		DOYLESTOWN HOSPITAL PAVILLION III	21.2
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327266	4/3/2019		2019 Seasonal	19.8
327267	4/3/2019		Various Jobs	18.8
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443513	4/10/2019		SEASONAL	12.0
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443514	4/10/2019		VARIOUS LEVEL 4	23.
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443516	4/10/2019		SEASONAL	21.
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327436	4/10/2019		M B & L Excavating - 2019 Seasonal	17.
443521	4/10/2019		M SEASONAL PRICE 2019	2:
327437	4/10/2019		M P & M Construction	13.
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443747	4/17/2019	7:37:00 A	M J.D JOBS	22

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	44.00
327647 4/17/2019 8:19:00 AM A & T Subaru	24.73
327648 4/17/2019 8:24:00 AM A & T Subaru	22.76
327649 4/17/2019 8:28:00 AM Various Jobs	22.76
327650 4/17/2019 8:32:00 AM A & T Subaru	13.69 22.68
443755 4/17/2019 8:33:00 AM NPWA E.BROAD ST.	21.67
327651 4/17/2019 8:34:00 AM A & T Subaru	a de distanção .
443756 4/17/2019 8:39:00 AM J.D JOBS	2.12
327652 4/17/2019 8:42:00 AM 2019 Seasonal	21.43
327653 4/17/2019 8:49:00 AM 2019 Seasonal	24.04
327654 4/17/2019 8:51:00 AM Costars 5610-36 E Rockhill	22.99
327655 4/17/2019 8:56:00 AM HD Paving	3.03
Company of the Compan	14.06
24	
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327766 4/24/2019 7:03:00 AM Doylestown Hospital	22.41
327767 4/24/2019 7:08:00 AM Doylestown Hospital	21.55
443931 4/24/2019 7:11:00 AM QUANTUM	9.3
443932 4/24/2019 7:15:00 AM VARIOUS JOBS	22.58
443933 4/24/2019 7:21:00 AM VARIOUS JOBS	22.45
327768 4/24/2019 7:24:00 AM Doylestown Hospital	21.77
327769 4/24/2019 7:29:00 AM Doylestown Hospital	23.08
327770 4/24/2019 7:33:00 AM Doylestown Hospital	21.14
443934 4/24/2019 7:38:00 AM VARIOUS JOBS	21.58
443935 4/24/2019 7:40:00 AM SR4013-BMP-ECMS#92741	21.84
443936 4/24/2019 7:41:00 AM JASON BRICKAJLIK	22.63
327771 4/24/2019 7:45:00 AM A.S. Paving	16.01
443937 4/24/2019 7:46:00 AM SEASONAL PRICING 2019	22.22
443938 4/24/2019 7:52:00 AM MASON LANDSCAPE & CONST.	3.66
327772 4/24/2019 7:54:00 AM 2019 Seasonal	20.97
443939 4/24/2019 7:56:00 AM JNS- SEASONAL PRICE 2019	22.5
327773 4/24/2019 7:59:00 AM A.S. Paving	20.95
327773 4/24/2019 7:59:00 AM A.S. Paving 443940 4/24/2019 8:01:00 AM OLD FORTY FT. RD. BRIDGE	20.28
327774 4/24/2019 8.03:00 AM PENNDOT LOT	21.87
1/21/2013 G.OO.OO ANY / CHINDO / LOC	21.94
472472013 8.11:00 AW VARIOUS JUBS	8.77
327776 4/24/2019 8:13:00 AM PennDOT Lot	21.95
327777 4/24/2019 8:17:00 AM PennDOT Lot	22.03
443942 4/24/2019 8:21:00 AM LIFE STORAGE	21.95

1	327778	4/24/2019	8:22:00 AM Doylestown Hospital	22.06
	- K	4/24/2019		21.76
			8:26:00 AM LIFE STORAGE	20.21
	327779	4/24/2019	8:27:00 AM Timber Creek	22.94
	443945	4/24/2019	8:29:00 AM VARIOUS JOBS	21.99
	443945	4/24/2019	8:31:00 AM 20 NOBLE ST. SELLERSVILLE	21.61
	327780	4/24/2019	8:32:00 AM Timber Creek	22.47
		4/24/2019	8:37:00 AM Timber Creek	22.97
	327781	4/24/2019	8:42:00 AM Doylestown Hospital	21.6
	327782	4/24/2019	8:43:00 AM JNS- SEASONAL PRICE 2019	20.87
	443947	4/24/2019	8:47:00 AM Cov-Wilson	22.02
	327783	4/24/2019	8:52:00 AM Doylestown Hospital	21.54
	327784		8:56:00 AM Doylestown Hospital	22.93
	327785		8:59:00 AM P & M Construction	8
	327786	4/24/2019	8:59:00 AIM F & IM CONSTITUTION	a., p. d
	38			77 4 F NOTATION
			the first of	
	444170	5/1/2019	7:06:00 AM OLD FORTY FT. RD. BRIDGE	21.84
	444171	5/1/2019	7:07:00 AM 261 TOWNSHIP LINE ROAD	15.7
	444172	5/1/2019	7:15:00 AM VARIOUS LOADS PICKED UP	19.71
	444173	5/1/2019	7:53:00 AM VARIOUS LEVEL 4	22.5
	444174	5/1/2019	7:56:00 AM A.S PAVING	3.95
	444175	5/1/2019	8:02:00 AM KRAGER TRUCKING	20.36
	327996	5/1/2019		22.72
	444176	5/1/2019		19.9
	327997	5/1/2019		23.79
	444177	5/1/2019		9.97
	327998	5/1/2019		22.98
	327999	5/1/2019	gr y	9.98
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		5 (0 (2010	7:03:00 AM 2019 Seasonal	20.25
	328112	5/8/2019	7:06:00 AM LIFE STORAGE OTTSVILLE	22.48
	444454		and the second s	22.68
	328113	5/8/2019	50105 2010	21.79
	444455	5/8/2019	The state of the s	22.15
	328114	5/8/2019		23.19
	328115			21.44
	444456		THE PROPERTY OF THE PROPERTY O	20.55
	444457	year a passes in a contract the		22.11
	328116		TO THE PROPERTY OF THE PARTY OF	20.03
	444458	2.4	and the second of the second o	23.08
	328117		and the same of th	22.43
	328118		Character Character	21.97
	328119		was an Australian March 1997 Allenda	22
	328120	_ Ann	A COMPANY TO BE A COMPANY TO B	21.6
	444459	5/8/2019	7:52:00 AM HORGAN JOBS LEVEL 4	Ba da (w 2

328121	5/8/2019	7:53:00 AM Sc	hmidt Paving - 2019	4.0
328122	5/8/2019	the second of the second of the second of	Contract to tent assessment and considerate	
328123	5/8/2019	The state of the contract of the state of th	The state of the s	22.6
444460	5/8/2019	and the community of the control of	ASONAL 2018 PRICING	20.8
328124	5/8/2019	Married Complete Community of the Commun	dden Meadows	22.5
444461	5/8/2019	APPL A COMPANY CONTRACTOR AND CONTRACTOR AND CONTRACTOR	W BRITIAN WAREHOUSE	21.6
328125	weeks the transmission in the at-	the man and an armon top or control	dden Meadows	23.1
444462	5/8/2019			2.88
444463	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	and the transfer that they may	ASONAL PRICE 2019	22.76
328126	5/8/2019		Market 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	21.55
328127	5/8/2019	- Proposition of Communication of	The same of the sa	22.04
328128	5/8/2019	a della conferenzazione dell'appropriate all'access reporte della fina		1.52
328129	5/8/2019	you are regarded described to the control of the co		22.49
444464	Total Laboration of the Contraction of the Contract	the Company of the contract of	ASONAL PRICE 2019	et skily take day on the
444465	Barn I also allegation in Colombinates and a significant debation	8:55:00 AM NP	plants the transfer and an arrangement of the second of th	21.03
and the second	3/0/2013	0.33.00 AIVI NF	WA C.BROAD 31.	9.98
30			and the same of th	
444676	5/15/2019	7:02:00 AM 201	L8 SEASONAL	23.01
444677	5/15/2019	7:09:00 AM SR4	013-BMP-ECMS#92741	13.83
444678	5/15/2019	7:18:00 AM NP	WA E.BROAD ST.	22.79
444679	5/15/2019	7:21:00 AM SEA	SONAL 2018 PRICING	20.97
444680	5/15/2019	7:23:00 AM SEA	SONAL 2018 PRICING	20.96
444681	5/15/2019	7:26:00 AM SEA		22.33
444682	5/15/2019	7:29:00 AM PIC	the same of the sa	0.87
444683	5/15/2019	7:48:00 AM E. S	CHMIDT PAVING-COD SEASONAL PRICE 2019	21.12
328392	5/15/2019	7:51:00 AM Var		1.54
328393	5/15/2019	7:57:00 AM Var	professor programmer.	22.72
444684	5/15/2019	7:59:00 AM VAR	RIOUS JOBS	22.67
328394	5/15/2019	8:03:00 AM Vari	The second section is the section of	22.98
444685	5/15/2019	8:04:00 AM VAR	The second secon	11.42
444686	5/15/2019	8:05:00 AM SEA		22.97
444687	5/15/2019	to the of the same comments of the same and the same of the same o	4 SQUIRES WAY LANSDALE	22.94
328395	5/15/2019		the control of the co	22.46
328396	5/15/2019	And the same and t	The second secon	21.97
328397		8:23:00 AM Vari	AUTO US B. WAS NAMED I.	22.98
444688	the little and approximately and approximately	8:24:00 AM 201	1.724	22.76
444689	5/15/2019	1 9 Mr. Harrings or Chapter assets 1 to the contract parties	CHMIDT PAVING-COD SEASONAL PRICE 2019	20.82
328398	*	8:30:00 AM Vari	Marring, pr. an. Agreeme group, graph & Ar.	- A
328399	5/15/2019		rence Site Contractors	23.99
444690	5/15/2019	8:36:00 AM KOP	LINERARCONIDY	21.94
444691	the state of the s	8:39:00 AM SEAS	Course with make 12 . Day attribute the same and any on the same and the	0.94
328400	5/15/2019	8:41:00 AM Law	rence Site Contractors	22.63
444692	5/15/2019	8:44:00 AM VAR	IOUS IORS	22.01
328401	5/15/2019	and the second second	rence Site Contractors	23.21
444693	5/15/2019	and the state of t	CHMIDT PAVING-COD SEASONAL PRICE 2019	21.98
777033	2/ 13/ 2013	6.37.00 HIVI E. 30	HIVID I PAVING-COD SEASONAL PRICE 2019	21.25

328437	5/15/2019	4:03:00 PM Various Jobs	14.21
	5/15/2015		
1			
444882	5/22/2019	7:11:00 AM LEVEL 4	17.13
444883	5/22/2019	7:14:00 AM MASON LANDSCAPE & CONST.	4.37
444884	5/22/2019	7:25:00 AM DUKE TRANSPORT-SEASONAL 2019	15.68
328615	5/22/2019	7:29:00 AM Various Jobs	8.11
328616	5/22/2019	7:35:00 AM Rodon	23.12
444885	5/22/2019	7:37:00 AM WEISS MKTS.	22.59
444886	5/22/2019	7:40:00 AM LEVEL 3 PICKUP	21.65
328617	5/22/2019	7:42:00 AM Various Jobs	20.12
328618	5/22/2019	7:43:00 AM 2019 Seasonal	3.06
328619	5/22/2019	7:52:00 AM Rodon	22.02
444887	5/22/2019	8:02:00 AM VARIOUS LEVEL 3	14.96
328620	5/22/2019	8:06:00 AM Rt 113	22.54
444888	5/22/2019	8:21:00 AM CARVERSVILLE FARM FOUNDATION	23.32
444889	5/22/2019	8:37:00 AM VARIOUS LEVEL 4	22.44
444890	5/22/2019	8:38:00 AM ANDREW ROTMAN	3.9
328621	5/22/2019	8:41:00 AM Rodon	21.98
328622	5/22/2019	8:42:00 AM New Britain Warehouse	3.04
444891	5/22/2019	8:48:00 AM SEASONAL PRICE 2019	23.02
328623	5/22/2019	8:57:00 AM Various Jobs	10.42
19			
			ندر سا
328711	5/29/2019	7:07:00 AM McDonald's Quakertown	22.18
328712	5/29/2019	7:12:00 AM McDonald's Quakertown	22.12
445026	5/29/2019	7:12:00 AM DUKE TRANSPORT-SEASONAL 2019	23.43
445027	5/29/2019	7:15:00 AM PICKED UP	1.28
328713	5/29/2019	7:16:00 AM McDonald's Quakertown	22.01
328714	5/29/2019	7:33:00 AM McDonald's Quakertown	23.04
328715		7:38:00 AM McDonald's Quakertown	21.99
328716	5/29/2019	8:12:00 AM Schmidt Paving - 2019	9.03
328717	- 1 1 1		9
328718	26 4.5 . 1994	8:18:00 AM 2019 Seasonal	20.91
445028	5/29/2019	8:22:00 AM LEVEL 3 PICKUP	22.4
328719	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	8:22:00 AM 2019 Seasonal	12.95
328720	. The second of the second	8:25:00 AM 2019 Seasonal	13.02
328721	(Maryon 1 1 ton 4: 7th 46:	8:34:00 AM D. Mallozzi	7.01
328722	The state of the s		8.02
445029	5/29/2019	8:47:00 AM KOPLIN MASONRY	1.03
16			

The state of the s	331951	8/7/2019	7:00:00 AM Weis Markets	22.64
	331952	8/7/2019	7:04:00 AM Hillstone	19.55
	447631	8/7/2019	7:07:00 AM 2018 SEASONAL	21.14
	331953	8/7/2019		22.35
special contracts	447632	8/7/2019	7:12:00 AM CHAPMAN RD.	21.46
	331954	8/7/2019	7:14:00 AM Weis Markets	22.8
	447633	8/7/2019	7:24:00 AM CHAPMAN RD.	21.9
	331955	8/7/2019	7:32:00 AM Weis Markets	21.94
	447634	8/7/2019	7:40:00 AM VARIOUS LEVEL 1	21.63
	331956	8/7/2019	7:44:00 AM Weis Markets	21.97
	331957	8/7/2019	7:46:00 AM Various Jobs	2.97
	447635	8/7/2019	7:46:00 AM JNS- SEASONAL PRICE 2019	22.41
	331958	8/7/2019	7:48:00 AM Schmidt Paving - 2019	11.99
	331959	8/7/2019	7:55:00 AM 2019 Seasonal	22.5
	447636		7:57:00 AM VARIOUS JOBS	1.83
	331960	8/7/2019	- Table 1 Lot 1 Lot pole Constraint Control 1 Providence	22.93
	331961	8/7/2019	8:05:00 AM Weis Markets	22.55
	447637	or direct relation (55,844,511 P.T.	8:06:00 AM VARIOUS	20.38
Si + 5#	447638	The second section of the sect	8:07:00 AM CHAPMAN RD.	21.78
	447639	A STATE OF THE STA	8:07:00 AM DOYLESTOWN HOSPITAL PAVILLION III	22.14
	447640	MAY S THE STREET STREET, STREE	8:08:00 AM VARIOUS LEVEL 4	21.84
	447641	www.heilaninimahautomorrous	8:09:00 AM VARIOUS	20.04
	447642	A STOCKER OF THE SECOND AND THE	8:11:00 AM JNS- SEASONAL PRICE 2019	
	447643	a contract to the contract to	8:13:00 AM PINNACLE PAVEMENT LLC	22.66
	331962	8/7/2019	8:14:00 AM Weis Markets	1.51
	447644	8/7/2019	8:22:00 AM VARIOUS	22.24
	331963	Committee of the second of the second of	8:23:00 AM Weis Markets	18.21
	447645	8/7/2019	8:25:00 AM VARIOUS LEVEL 1	22.74
• •	331964	Draw Tree & Agence	8:29:00 AM Weis Markets	21.7
	447646	8/7/2019	8:31:00 AM JNS- SEASONAL PRICE 2019	21.99
	447647	orangement of the second	8:32:00 AM VARIOUS	22.56
	331965	Carlot Street or A. spiral	8:32:00 AM DiPalantino Contractors, Inc.	17.68
	447648	8/7/2019	8:40:00 AM VARIOUS LEVEL 4	13.99
	447649	- and referred to colours a company to the colour	Compression of the contract of	19.25
ne	-	8/7/2019	8:43:00 AM VARIOUS	20.22
	331966	8/7/2019	8:48:00 AM Weis Markets	22
	331967	8/7/2019	8:51:00 AM Various Jobs	9
	447650	8/7/2019	8:57:00 AM CHAPMAN RD.	22.02
	447651	8/7/2019	8:58:00 AM VARIOUS LEVEL 4	21.1
	331968	8/7/2019	8:58:00 AM Weis Markets	22.48
	39			-
	447938	8/14/2019	7:06:00 AM VARIOUS JOBS	9.98
	332352	8/14/2019	7:07:00 AM Pennridge Business Park	22.61
	332353	8/14/2019	7:12:00 AM Various Jobs	22.1
	447939	8/14/2019	7:12:00 AM VARIOUS JOBS	21.27
	332354	8/14/2019	7:16:00 AM Various Jobs	22.53

	222255	0/14/2010	7:21:00 AM	Pennridge Business Park	22.2
	332355	8/14/2019	2 2 2 4 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	CHAPMAN RD.	21.63
	447940	8/14/2019	7:26:00 AM	grade agreement of the second	22.96
	professor intriguigation places that the second	8/14/2019	7:29:00 AM		0.92
	en e	8/14/2019	7:30:00 AM		21.46
	para representation of the same	8/14/2019	and the automotive age or entere efficiely	VARIOUS JOBS	21.57
	447942	8/14/2019			22.75
		8/14/2019		Pennridge Business Park	21.06
		8/14/2019	4 mg 4 mg 4 mg 1 mg 1 mg 1 mg 1 mg 1 mg	ALBRECHT EXCAVATING	21.4
	447944	8/14/2019		VARIOUS JOBS	21.04
	332359	8/14/2019	7:41:00 AM	as disputation to the contract of the contract	21.88
	per a bed does	8/14/2019		2018 SEASONAL	4.4
	447946	8/14/2019	The second secon	NEW BRITAIN TWP. CULVERTS	21.77
	447947	8/14/2019		K SQUARED LANDSCAPE LLC	4.43
	332360	8/14/2019	7:51:00 AM	The Control of State of Control o	23.98
	447948	8/14/2019	Commence of the Commence of th	ECMS102837 RAILROAD AVE.	20.58
	447949	8/14/2019	7:56:00 AM	promote agreement and the contract of the cont	4.98
	447950	8/14/2019	8:00:00 AM	VARIOUS JOBS	21.26
	concern a Warman Colon of A	8/14/2019	8:02:00 AM	Control State where many appropriated with a con-	20.87
**	447952	8/14/2019	8:07:00 AM	ECMS102837 RAILROAD AVE.	20.21
	womaning Co., water, and 3 retir	8/14/2019		MIDDLE RD-DUBLIN VLG.	20.8
	332361	8/14/2019	8:15:00 AM	RFQ561036ITQ 19-01 FOB Bucks 9.5mm	6
	Processor Miles and Control	8/14/2019	8:22:00 AM	NEW BRITAIN TWP. CULVERTS	22.38
	and the second second	8/14/2019	8:23:00 AM	SEASONAL PRICE 2019	23.08
	4 of 1 1 mm	8/14/2019		SEASONAL 2018 PRICING	20.78
	447957	Will all annual	8:30:00 AM	And the state of t	20.06
	447958		-	VARIOUS JOBS	20.95
	447959	A second		847 BETHLEHEM PIKE	22.96
	447960	1 10 1 10 10 10		ECMS102837 RAILROAD AVE.	20.05
	447961	A PART OF	make a control of the	NEW BRITAIN TWP. CULVERTS	21.64
	447962			VARIOUS - LEVEL 4	21.77
		0/14/2015	0.50.00 /		
	35				
	332777	8/21/2019	7:02:00 AM	St. Johns Church	23.06
	332778	8/21/2019		Towamencin	22.06
	332779	and the second		2019 Seasonal	23.03
	448155	prince in the con-		CHAPMAN RD.	22.06
	332780	and the second	- /	Various Jobs	21.95
	448156			VARIOUS LEVEL 4	21.99
	332781			Towamencin	22.92
	448157			BUX MONT JOBS	8.79
	332782	94 P F F F F F F F F F F	Character of the Common	Towamencin	22.86
	448158			SEASONAL 2018 PRICING	21.31
	and the second of the	***	e par payer come or regarded.	COSTARS 5610-15 HILLTOWN TWSP.	9.5
	448159	വി. വി		Various Jobs	21.49
	332783	100	and the transfer of the property	VARIOUS LEVEL 3	20.99
	448160	The same of a contract of	And the second second	Costars 5610-36 Salford S Dietz Mill	23.01
	332784	8/21/2019	0'12'00 WIM	Costats sorto so sanota s piete	_310_

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to de la	332786	8/21/2019	8:18:00 AM Costars 5610-36 Salford S Dietz Mill	24.02
Constant	332785	8/21/2019	and the common page agreement to the common of the common	3.59
	332787	8/21/2019	and the second s	18.93
	332788	8/21/2019	AND TO ADMINISTRATION OF THE PARTY OF THE PA	6.01
	332789	8/21/2019		23.3
	448161	8/21/2019	to the territorial and the second of the sec	3.29
	332790	8/21/2019	THE EAST WARTER THOMPSON TO COMMENT	22.97
	332791	8/21/2019	* 32 1	22.75
	22			22.73
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	333137	8/28/2019	7:07:00 AM Pine Run Rd	22.35
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	333139	8/28/2019	7:42:00 AM Pine Run Rd	22.73
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	333141	8/28/2019	8:00:00 AM E Saw Mill Rd	21.73
	333142	8/28/2019	8:04:00 AM E Saw Mill Rd	22.97
	448301	8/28/2019	8:07:00 AM GENERAL SERVICES-PARKS	4.36
	333143	8/28/2019	8:09:00 AM E Saw Mill Rd	22.97
	333144	8/28/2019	8:15:00 AM Pine Run Rd	22.54
	333145	8/28/2019	8:17:00 AM CoStars 5610-36 Hilltown	9.49
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	333147	8/28/2019	8:23:00 AM CoStars 5610-36 Hilltown	9.52
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	333148	8/28/2019	8:26:00 AM CoStars 5610-36 Hilltown	9.01
	333149	8/28/2019	8:29:00 AM 2019 Seasonal	15
	333150	8/28/2019	8:33:00 AM Various Jobs	18.03
	448303	8/28/2019	8:37:00 AM JOHN MADGIC	1.05
	333151	8/28/2019	8:39:00 AM Pine Run Rd	22.46
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	448304	8/28/2019	8:57:00 AM DOYLESTOWN HOSPITAL	19.01
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	448478	9/4/2019	7:03:00 AM 2018 SEASONAL	22.61
	333435	9/4/2019	7:06:00 AM Court St. Doylestown	23.98
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	448480	9/4/2019	7:24:00 AM DOYLESTOWN HOSPITAL PAVILLION III	18.67
	333436	9/4/2019	7:27:00 AM Various Jobs	22.98
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	448888	9/11/2019	8:50:00 AM	COSTARS 5610-15 LANSDALE BORO	8.97
	333825	9/11/2019	8:51:00 AM	Various Jobs	14.48
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	448889	9/11/2019	8:55:00 AM	REGENCY @ HILLTOWN	21.42
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	449212	9/18/2019	8:56:00 AM	MOORE'S EXCAVATING	21.54
	334386	9/18/2019	8:57:00 AM	Various Jobs	21.96
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	449485	9/25/2019	7:03:00 AM	VARIOUS JOBS	21.21
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í	334930	9/25/2019	7:13:00 AM	Various Jobs	22.39
	449488	9/25/2019	7:18:00 AM	CHAPMAN RD.	22.29
	449489	9/25/2019	7:19:00 AM	MASON LANDSCAPE & CONST.	2.71
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	449490	9/25/2019	7:22:00 AM	WAWA HILLTOWN	23.03
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	334934	9/25/2019	7:38:00 AM	Spring House	21.96
	449491	9/25/2019	7:43:00 AM	PENNLAND FARM BASIN CONVERSION	22.38
	449492	9/25/2019	7:46:00 AM	HORGAN JOBS LEVEL 4	21.64
	334935	9/25/2019	7:46:00 AM	Various Jobs	23.99
	334936	9/25/2019	7:50:00 AM	Various Jobs	23.04
	449493	9/25/2019	7:52:00 AM	VARIOUS JOBS	21.16
	334937	9/25/2019	7:55:00 AM	Spring House	22.01
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	334940	9/25/2019	8:11:00 AM	Various Jobs	21.98
	334941	9/25/2019	8:16:00 AM	Various Jobs	22.48
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Attachment ${\bf E}$ Wetland and Waters Delineation Map/Verification Letter





610.762.0990 484.661.6209



3282 Hope Drive Emmaus, PA 18049



www.valenv.com

Via Email (sdrumbore@hkgroup.com)

February 13, 2020

The H&K Group Engineering & Environmental Services Division 2052 Lucon Road P.O. Box 196 Skippack, PA 19474

Attn:

Scott Drumbore, P.E., Division Manager

Re:

Wetland/Watercourse Boundary Verification

Center Valley Materials

Springfield Twp., Bucks Co., PA

Valley Environmental Services, Inc. Job No. 18-227

Dear Mr. Drumbore:

Valley Environmental Services, Inc. has reviewed the wetland and watercourse boundaries illustrated on the plans entitled "Wetlands & Waters Delineation Map", sheet numbers 1-3 of 3, dated February 12, 2020, prepared by the H&K Group's Engineering & Environmental Services Division. This letter verifies the above referenced plans accurately represent those portions of the property field delineated on June 10-13, 2019.

Please be aware that any encroachment or impacts to the delineated portion(s) of the property may require permitting from the Pennsylvania Department of Environmental Protection and/or the United State Army Corps of Engineers.

Should you have any questions, or require additional information, please do not hesitate to contact me.

Sincerely,

Valley Environmental Services, Inc.

Jason J. Mease, PWS

Professional Wetland Scientist/Principal





610.762.0990 484.661.6209



3282 Hope Drive Emmaus, PA 18049



www.valenv.com

Via Email (sdrumbore@hkgroup.com)

February 13, 2020

The H&K Group Engineering & Environmental Services Division 2052 Lucon Road P.O. Box 196 Skippack, PA 19474

Attn: Scott Drumbore, P.E., Division Manager

Re: Observed Wetland Hydrology

Center Valley Materials
Springfield Twp., Bucks Co., PA

Valley Environmental Services, Inc. Job No. 18-227

Dear Mr. Drumbore:

As requested, Valley Environmental Services, Inc. (VES) has completed numerous thorough evaluations of the above referenced property. The purpose of these evaluations was to identify and delineate (flag) all encountered regulated aquatic resources (e.g. wetlands and watercourses). Site evaluations were conducted on May 29th and 30th of 2018 and June 10th, 11th, 12th, and 13th of 2019. Utilizing the guidelines set forth in the *Corps of Engineers Wetland Delineation Manual* Y-87-01 (Environmental Laboratory, January 1987-Final Report) and their applicable *Eastern Mountains and Piedmont Regional Supplement* Version 2.0 (U.S. Army Corps of Engineers, April 2012) the vegetation, soils, and hydrology throughout the property were examined to determine the presence/absence of regulated features.

While completing the above referenced sitework numerous habitat specific characteristics were encountered. Based upon a combination of biologic, geomorphologic, and hydrologic traits observed it is VES's professional opinion that Wetlands A, B, F, G, J, I, and H, as identified on the plan entitled "Wetlands & Waters Delineation Map" (sheet number 1 of 3, dated February 12, 2020, prepared by the H&K Group's Engineering & Environmental Services Division) are primarily fed via perennial sources of hydrology like groundwater (springs) and baseflow. Alternately, Wetlands C, D, and E appear to be sustained primarily through ephemeral sources, like overland flow, in response to seasonal and annual fluctuations in precipitation.

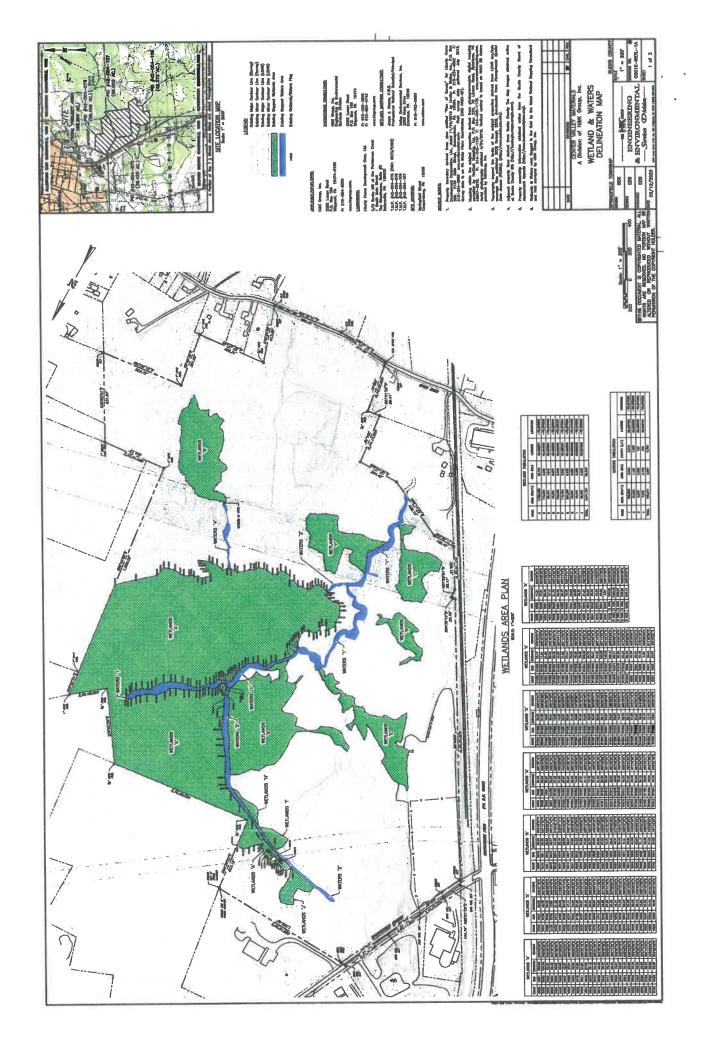
Should you have any questions, or require additional information, please do not hesitate to contact me.

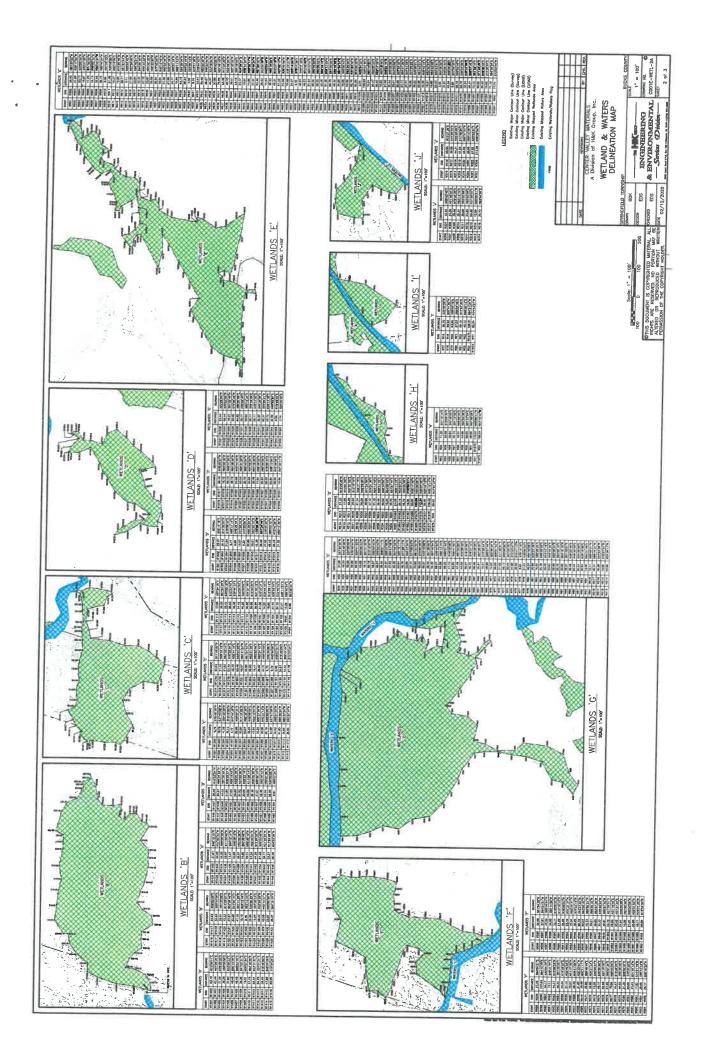
Sincerely,

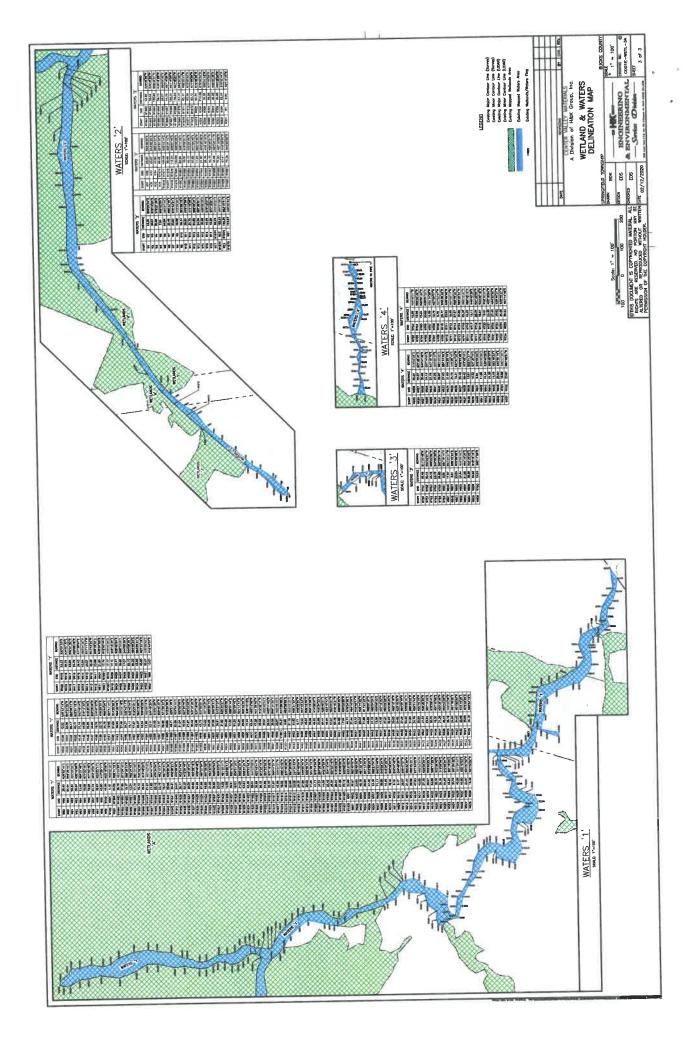
Valley Environmental Services, Inc.

Jason J. Mease, PWS

Professional Wetland Scientist/Principal







Center Valley Materials
Springfield Township, Bucks County – Conditional Use Application

Attachment F **Groundwater Model Report**

PRELIMINARY GROUNDWATER MODEL REPORT CENTER VALLEY MATERIALS SPRINGFIELD TOWNSHIP **BUCKS COUNTY, PENNSYLVANIA**

Prepared For:

The H&K Group, Engineering & Environmental Services Division 2052 Lucon Road P.O. Box 196 Skippack, PA 19474

Prepared By:

V.F. Britton Group, LLC 326 Conestoga Road Wayne, PA 19087 (610) 964-1462

March 12, 2020

Val F. Britton, P.G. **Technical Consultant**





VAL F. BRITTO

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Figure 7 - Model Calibration

Figure 8 – Simulation of Existing Groundwater Elevation Conditions

Figure 9 - Simulation - Dewatering of Northern Pit Groundwater Elevation Contour (400

Foot Bench Elevation)

Figure 10 - Simulation - Dewatering of Southern Pit Groundwater Elevation Contour (400 Foot Bench Elevation)

Figure 11 – Simulation -Northern Pit Drawdown (400 Foot Bench)

Figure 12 – Simulation -Southern Pit Drawdown (400 Foot Bench)

1.0 General

1.1 Background

The proposed Center Valley Materials surface mining operation is located south of Springfield Street and east of Route 309 (Bethlehem Pike) just south of Coopersburg, in Springfield Township, Bucks County, Pennsylvania (site). The proposed surface mining operation will consist of two quarry pit areas one located in the northern portion of the site (Northern Pit) and one located in the southern portion of the site (Southern Pit). Figure 1 presents the locations of the site and the two pit areas.

It is anticipated that one pit will be mined to completion prior to moving to the second pit location. The current ground elevation at the mining locations is approximately 600 feet mean sea level (ft-msl) and it is proposed that both pits will be mined to a bench elevation of 400 ft-msl. Both pit locations exist in a relatively flat area underlain by poorly drained diabase bedrock material and as a result, wetland areas exist proximate to the pit locations. A general site layout is provided on Figure 2.

Although no groundwater elevation monitoring wells have been installed on the site area, it is anticipated that a shallow water table exists, likely perched on the underlying poorly drained diabase geology.

The purpose of the preliminary groundwater model discussed in this report is to provide a preliminary numeric representation of the hydro-geologic conditions existing at the site and the surrounding area based on the existing site conceptual model. The model will allow the evaluation of groundwater flow at the site and in the surrounding areas under existing site conditions through numeric simulations based on available hydrogeological published data. The preliminary model will then be used for initial predictive evaluation of the groundwater elevation and zone of influence of the proposed active mining operations at the maximum mining limits (400-foot bench elevation). This preliminary model provides an initial evaluation of the hydrogeological conditions and is based on available published hydrogeological material that characterizes the site conditions. Once site-specific data is collected (i.e. monitoring well water level data, stream flow data, etc.), the model and results will be refined based on the site-specific data.

Limited hydro-geologic data has been collected by others related to site characteristic data; however, the data that has been collected has been relied upon for the construction of the groundwater model. It is not the intent of this groundwater model document to describe the methods used to collect this data, however, in some cases a brief explanation of the quality of the data is discussed where interpretation is warranted.

Groundwater Modeling Systems (GMS) software, Version 10.1, developed by the United States Department of Defense and distributed by Aqueveo, Inc. was utilized in the development of the groundwater model for the site. This modeling software consists of numerous modules that are interfaced to allow more accurate representation of hydro-

geologic conditions and greater flexibility in simulating and evaluating flow conditions on the site and surrounding area.

As discussed above, the majority of the preliminary groundwater model provided in this document is based on available hydrogeological published information with very little site-specific data. Once site-specific data is collected, this preliminary groundwater model and results will be refined with the data.

The "site" generally encompasses the entire region of the groundwater model that incorporates numerous properties within the general drainage basin of the site.

It is not the intent of the groundwater model to solely define the hydrogeologic characteristics that exist at the site, but rather this preliminary groundwater model is intended to be used as a preliminary screening tool to evaluate anticipated drawdown conditions associated with the proposed quarry operations.

Figures that have been included as part of this report are provided in an 11 x 17-inch paper size format and are in color allowing the data to be graphically presented. Black and white copies and/or smaller paper size copies of the figures may not present the data in the clarity originally intended.

2.0 Model Construction

2.1 General

The collection and or evaluation of all data desired for a particular investigative purpose may not be possible due to economics and/or logistic limitations. For this reason, some assumptions relative to the site's geologic or hydro-geologic characteristics have been made during the development of this preliminary groundwater model. However, all of the assumptions have been based on sound and accepted geologic and hydro-geologic theory and are identified when utilized.

The model was constructed in three stages. The first stage consisted of developing a three-dimensional conceptual model representing the physical characteristics of the site. The second stage consisted of converting the three-dimensional conceptual model into a numeric model for calibration. MODFLOW 20050, a finite difference model, was utilized for the numeric model. The model was constructed as a steady-state model, which allows the input data to be interpolated through numerous iterations to solve the finite difference equation. The third step consisted of running a predictive flow simulation to represent maximum pumping conditions at the site under average recharge conditions. The modeled simulations presented in this report were run under steady state conditions.

2.2 Numeric Flow Model Construction

Boundary Conditions

The boundary conditions of the model are provided on Figure 3. The selection of the model boundary was based on isolating the groundwater drainage basin that the site is within: groundwater that flows into the basin interacts with the model domain and becomes incorporated into the water budget and water that flows outside of the basin does not interact with the model domain and is not part of the water budget and; therefore, not part of the model. However, it should be noted that the Northern Pit area is located very close to the northern boundary of the regional groundwater divide (watershed boundary) and; therefore, the model domain was expanded in a northern direction into the neighboring watershed to prevent the potential for the Northern Pit dewatering simulation from interacting with the model boundary.

The general model area (model domain), with the exception to the north as noted above, is bordered by MODFLOW "no flow" boundaries that represent watershed divides. MODFLOW drain boundaries represent locations where surface water drainage would simulate the removal of water from the model as base flow. Drain boundaries were used to simulate the tributaries that likely only receive base flow drainage and surface water would not likely enter the underlying porous media.

The elevations of the surface waterways (drain nodes) were based on the United States Geologic Survey (USGS) topographic quadrangle map of the area. The elevations were estimated from the USGS maps and then 2.0 feet were subtracted from the estimated surface water elevation at each node location to estimate the bottom of the creek bed elevation. The bottom of the creek bed elevation was used in the model as the node elevation. The node locations are presented on Figure 3.

Surface Water

As discussed above, tributaries were mapped as MODFLOW drain arcs. The elevations of the drain nodes were based on the United States Geologic Survey (USGS) topographic quadrangle map of the area as discussed above. The node locations are presented on Figure 3.

The conductance values assigned to the surface water bodies were determined from the estimated creek dimensions and the estimated hydraulic conductivity of the creek bed material. Conductance is the leakage of water through the stream bed material that can discharge as base flow to the stream or leak back into the aquifer as recharge. Conductance is calculated by the product of the creek width and the hydraulic conductivity divided by the creek bed thickness. This provides a conductance value per unit distance (per foot) for the stream bed material. When this value is assigned to MODFLOW, the unit distance is multiplied by the length of the stream bed material in each cell of the model and the conductance of each cell is assigned to the MODFLOW model. A conductance value of 5.0 feet²/day/foot was used for the tributaries within the model domain.

Model Grid

Based on site specific geologic information collected from available publication data (Sloto and Schreffler, 1994, Plate 1), it is apparent that the site exists within a diabase intrusion with no specific preferential groundwater flow direction. For this reason, no specific grid orientation was assigned and the model grid was oriented north to south.

A general grid spacing of approximately 100 by 100 feet was assigned to the entire domain of the model. The model boundary conditions (drains package) were assigned to the grid. The general model grid is presented on Figure 4.

Model Layers and Geologic Characteristics

Two major geologic units exist within the model domain and consist of a diabase intrusion (diabase) and the Triassic age Brunswick Formation. The distribution of these formations within the model domain is presented on Figure 5. The distribution of these geologic units is based on site specific data obtained from publication data (Sloto and Schreffler, 1994, Plate 1). The current site topography has been superimposed on Figure 5 presenting the current site conditions used in the model.

Based on publication data (Sloto and Schreffler, 1994, pp. 17-20), the upper 100 feet of consolidated rock have the highest permeability when compared to the underlying rocks.

Generally, as stated in the published data, based on water-bearing fractures encountered during well installations, there are 4 hydrogeological zones underlying the site area (Sloto and Schreffler, 1994, p. 36). With depth (deeper than 400 feet), these water-bearing fractures eventually disappear reducing the permeability of the deeper aquifer. The diabase has very limited water-bearing capacity below 50 to 100 feet (Sloto and Schreffler, 1994, p.17-20).

For the purpose of the model construction, the model was assigned five MODFLOW layers to reflect the four upper hydrogeologic zones; 0 to 100 feet (layer 1), 100 to 200 feet (layer 2), 200 to 300 feet (layer 3), 300 to 400 feet (layer 4), and the base of the model with no permeability (layer 5). Figure 6 presents the general layer configuration of the model along with hydraulic parameters used in the model and discussed below.

Hydraulic Conductivity Assignment

Hydraulic conductivities were obtained from publication data (Sloto and Schreffler, 1994, pp. 36-41, and Reese and Risser, 2010, Plate 3); however, adjustments were made during the calibration process to the initial values to obtain an appropriate calibration of the model. Table 1 provides a summary of the key hydrogeological publication data and the actual values used in the model for calibration purposes. In addition, the hydrogeological assignments for each layer are provided on Figure 6.

Vertical anisotropy ratios were assigned to the model layers. The vertical hydraulic conductivity ratios assigned to all of the deeper diabase layers was 0.5 based on publication data (Senior 1999). It should be noted that this was not a sensitive parameter in the model.

Recharge

Groundwater recharge is based on annual precipitation, infiltration rates, stream base flow rates, and evapotranspiration rates. As a general "rule of thumb", recharge is approximately 1/3 of the actual precipitation that occurs in relatively flat and porous terrain. Initial recharge values were obtained from publication data (Sloto and Schreffler, 1994, pp. 52-54, and Reese and Riser, 2010, Plate 3) and were refined during the calibration process. Based on the publication data, recharge in the area of the site ranges from 10.0 to 12.0 inches per year. Specifically, in the area of the diabase, recharge was reported to be approximately 2.0 inches per year and the Brunswick Formation area was reported to be approximately 8 to 12 inches per year. The final mean recharge values used over the entire domain of the model based on the model calibration was 2.0 inches per year for the diabase and 8.5 inches per year for the Brunswick Formation.

A sensitivity analysis of average recharge was conducted to better understand the impact that this parameter has on the groundwater movement on the site. Average recharge and hydraulic conductivity were used for the sensitivity analyses. Higher and lower values of recharge were evaluated. It was determined that the model was very sensitive to recharge: the higher values caused flooding in the model in areas that none was observed, and the

lower recharge values resulted in "dry cells" in the model where groundwater was known to exist. Additionally, the sensitivity analysis was compared to the residual error between the observed groundwater elevations and the simulated groundwater elevations.

2.3 Numeric Flow Model Calibration

General

Calibration refers to the demonstration that the model is capable of producing field measured heads and flows. Calibration can be evaluated both qualitatively and quantitatively; however, even in a quantitative evaluation, the judgment of when the fit between model and reality is satisfactory is a subjective one (Anderson, 1992, pp. 223-246).

The groundwater model was calibrated to estimated groundwater elevation (head) data collected from two site boreholes, two residential wells, six well location data points provided by the Pennsylvania Groundwater Information System (PAGWIS), and three control points (CP) based on stream elevations on USGS topographical quadrangle maps. At this time (preliminary groundwater model development), no site-specific groundwater elevation monitoring wells have been installed on the site for characterization purposes. Once site monitoring wells are installed and data are collected, the model will be recalibrated with the site-specific data.

Model Calibration (Head Elevation)

Groundwater elevation data obtained from the sources discussed above was used to calibrate the preliminary groundwater model. Table 2 provides a tabulation of the groundwater elevation data used for each of the calibration locations. It should be noted that the data obtained provides a general groundwater elevation; however, does not provide the same quality of data that long-term site-specific monitoring well data would provide. Once site-specific data is collected, the groundwater model will be re-calibrated.

During the calibration process, a sensitivity analyses of the recharge values and the hydraulic conductivity values were conducted to identify the most unique parameter values to best match the calibration targets (head elevations at the calibration points). The sensitivity analysis is provided on Table 3. The final hydrogeological values used in the model are discussed above and provided on Figure 6 and Table 1.

The result of the calibration (residual error) is presented in tabular format on Table 2 and is graphically presented on Figure 7. Based on a reasonable distribution of calibration points (groundwater head values) on both sides of the perfect fit line (see Figure 7), a reasonable calibration was achieved using the available data. Based on this calibration, a mean error over the domain of the model of 5.02 feet was achieved which equates to an approximate normalized error of 12.2 % (mean error divided by the range of water elevation within the domain of the model). Additional calibration statistics are provided on Figure 7 and Table 2.

Sensitivity Analyses

Several sensitivity evaluations were conducted on the hydraulic parameters input into the model. The sensitivity analyses allow key parameters of the model to be adjusted independently of the other parameters to evaluate the sensitivity of each of the parameters within the model. Generally, the purpose of the sensitivity analyses confirms the uniqueness of the set of hydrogeologic parameters used in the model. This prevents the use of model boundary conditions that allow broad ranges of parameter values that are non-unique to a specific site.

During the calibration, the recharge values were changed while the average hydraulic conductivity values for each of the four geologic zones were held constant. These values were derived through a trial and error process. Once the best quantitative calibration was obtained, a sensitivity analysis of the parameters was conducted.

Flow Budget

The flow budget of the MODFLOW model was evaluated to determine if reasonable inflows and outflows of the model had been achieved. Based on a conceptual understanding of the site's hydrologic cycle, it was apparent that the aquifer on the site was recharged from precipitation. Groundwater was lost from the aquifer through drainage into surface water creeks (drains). Results of the flow budget are presented on Table 4.

Based on the results of the flow budget, it is evident that the inflow of water into the model domain closely matches the outflow of water from the model domain suggesting a reasonable water budget balance.

3.0 Simulated Groundwater Flow

3.1 Existing Site Groundwater Elevation Conditions

Based on the calibrated groundwater model, Figure 8 presents the existing groundwater flow elevation contours for the existing site static non-pumping conditions on a regional level. A mean recharge value of 8.5 inches per year was used for the Brunswick Formation and mean recharge value of 2.0 inches per year was used for the diabase in this simulation.

3.2 Simulation of Maximum Pit Dewatering Conditions (Bench – 400 ft-msl)

It is anticipated that one pit location will be mined to its entirety prior to mining the second pit. For this reason, the dewatering simulations for each pit (Northern and Southern Pit) were simulated separately and not at the same time. The dewatering simulations represented the maximum drawdown for each pit which correlates to a final bench elevation of 400 ft-msl. For the dewatering simulations each of the pits were dewatered to the maximum depth of 400 ft-msl. The bench configurations for each pit were based on the mining bench plan (50-foot depth expansions down to 400 ft-msl). The pit dewatering was simulated with MODFLOW drains over the area of the benches at the appropriate bench elevation (600 ft-msl down to 400 ft-msl). This configuration allowed the model to simulate the proposed pit configurations under the maximum dewatering scenario.

Figures 9 and 10 provide the groundwater elevations for the dewatering of the Northern Pit and the Southern Pit respectively. Figures 11 and 12 provide the associated simulated drawdown for the Northern Pit and the Southern Pit respectively. The drawdown was obtained from subtracting the maximum pumping groundwater elevation contours (Figures 9 and 10) from the static groundwater elevation contours (Figure 8) within the model software.

A is evident on drawdown Figures 11 and 12, very little dewatering impact is occurring due to the low permeability of the diabase material. Both the Northern Pit and Southern Pit have less than 1000 feet of radial expansion to the 10-foot drawdown mark. Additionally, the deeper the pits are expanded into the diabase material, tighter and less fractured diabase is encountered with very little water-bearing capacity. It is anticipated that the majority of the groundwater impact will occur in the first 50-foot bench expansion since this is the most weathered and water-bearing zone in the diabase. Based on the model simulations, the Northern Pit is estimated to yield an average 21 gallons per minute (gpm) during the maximum dewatering process. The Southern Pit is estimated to yield an average of 17 gpm during the dewatering process.

It should be noted that the model simulations tend to over-predict the drawdown impact from the dewatering operations due to the steady-state nature of the model simulations. The model simulations are 100% efficient in calculating the water budgets when in

reality, dewatering occurs over time and is not instantaneous. This would result in a smaller zone of influence than is presented in this report.

4.0 Summary and Conclusions

Based on the results of the groundwater model simulation discussed in this document, the following conclusions have been reached:

- The proposed Northern and Southern Pit locations are situated on diabase geologic material that has very limited water-bearing capacity. Furthermore, this capacity decreases with depth.
- A very limited horizontal zone of influence is expected to occur from the proposed pit locations under the maximum dewatering scenario (Bench elevations at 400 ft-msl). Less than a 1000-foot radius was calculated for both pit locations and is likely to be much less since model simulations tend to over predict drawdown.
- Vertical expansion of the pits is not likely to increase the zone of influence since the water-bearing capacity of the diabase material decrease with depth. Only the upper 50 to 100 feet are reported to have very limited water-bearing capacity with almost no capacity below 100 feet.
- Dewatering rates are expected to range from 17 to 21 gallons per minute at the deepest pit depth (400 ft-msl) with the majority of the dewatering yield coming from the upper 50 feet.

5.0 Limitations

The modeling in this report was performed using a commercially available software package (Groundwater Modeling System-GMS, Version 10.1 developed by the United States Department of Defense) designed to simulate groundwater flow. Where available, actual data from the site was utilized to calibrate the models and develop the graphical representations presented in this document. In other instances, assumptions were necessary to complete the model and limitations associated with the site data result in a level of uncertainty in the model predictions. Therefore, the results of the model predictions should be independently evaluated using actual site monitoring data.

The results of the model may differ from actual site conditions because of unknown subsurface conditions. The results of the models presented in this document shall not be construed to create any warranty or representation with regard to the site. The conclusions presented in this report were based on the services described, and not on scientific tasks or procedures beyond the described scope of services.

6.0 References

Anderson, M.P., Woessner, W. W., 1992, Applied Groundwater Modeling – Simulation of Flow and Advective Transport, Academic Press, Inc., pp. 223-246.

Reese, S.O., and Risser, D.W., Summary of Groundwater-Recharge Estimates for Pennsylvania Water Resource Report 70, Pennsylvania Geologic Survey, 2010.

Senior, L.A., and Goode, D.J., Ground-Water System, Estimation of Aquifer Hydraulic Properties, and Effects of Pumping on Ground-Water Flow in Triassic Sedimentary Rocks in and near Lansdale, Pennsylvania, U.S. Geologic Survey, Water-Resources Investigations Report 99-4228, 1999.

Sloto, R.A., and Schreffler, C.L., Hydrogeology and Ground-Water Quality of Northern Bucks County, Pennsylvania, U.S. Geological Survey, Water-Resources Investigations Report 94-4109, 1994.

TABLES

TABLE 1

Key Hydraulic Parameters Preliminary Groundwater Model Center Valley Materials Springfield Township Bucks County, Pennsylvania

Source	Published Value(s)	Model Value	Comments
lydraulic Conductivity			
USGS - Water-Resources	0.475 ft/day Brunswick	0.328 ft/day Brunswick	Assumes aquifer is approximately 400 feet thick as discussed in publication relative to available fracture zones.
USGS - Water-Resources Investigations Report 94-4109 (Sloto, 1994, pp.17, 35-41)	0.1 ft/day Diabase	0.1 ft/day Diabase	Assumes aquifer is approximately 400 feet thick as discussed in publication relative to available fracture zones.
Anisotropy Ratios (Preferential Flow			- I I I I I I I I I I I I I I I I I I I
USGS - Water-Resources Investigations Report 99-4228 (Senior, 1999)	20 to 1	1.0	Based on diabase characteristics and site location within diabase area.
Recharge			
USGS - Water-Resources Investigations Report 94-4109 (Sloto, 1994, pp.52-53)	2.0 in/year Diabase 8.5 in/year Brunswick	2.0 in/year Diabase 8.5 in/year Brunswick	Based on statics and model calibration.
Summary of Groundwater- Recharge Estimates for Pennsylvania PAGS- Water Resource Report 70 (Reese and Risser, 2010, Plate 3)	10 to 12 inches per year	2.0 in/year Diabase 8.5 in/year Brunswick	Publication is regional and does no evaluate individual subwatersheds.

TABLE 2

Residual Error of Calibration Points Springfield Township Bucks County, Pennsylvania Center Valley Materials Calibration

	Description	Computed (Simulated) Groundwater Elevation	Observed Groundwater Elevation	Residual Error (feet)
CP-A	Surface Water Point	580	(feet-msl)	
CP-B	Surface Water Point	200	086	1.0
CP-3	Surface Water Boint	3/1	009	29.0
T	Service Water FUIII	296	598	Lc
CIN	Measured Residential Well	530	1100	6.2
HK-2	Measured Borehole	504	25/	27.2
TATOO	Measured Recidential Well	+60	909	12.7
HK-3	IDAA IBIIIDAGA BARAA AAAAAAAAAAAAAAAAAAAAAAAAAAAAAA	594	605	110
	iviedsured borenole	593	502	
5862	PAGWIS Well	601		-0.8
73443	PAGWIS WAII	700	586	-14.7
474788	PAGWIS Well	3/1	556	-15.0
1	III CAA CIAA CIA	603	586	1000
/339/	PAGWIS Well	621		-TP.6
73460	PAGWIS Well	777	628	7.1
72840	PAGWIC WAI	/10	630	12.9
	יייייייייייייייייייייייייייייייייייייי	637	212	

Mean Error:

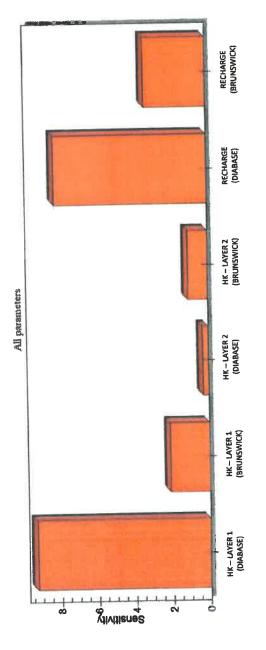
5.02 8.07 11.07 12.20% Absolute Mean Error:

Root Mean Square Error:

Normalized RMS:

TABLE 3

Parameter Sensitivity Center Valley Materials Springfield Township Bucks County, Pennsylvania



HK – HORIZONTAL HYDRAULIC CONDUCTIVITY

TABLE 4

Flow Budget Preliminary Groundwater Model Center Valley Materials Springfield Township Bucks County, Pennsylvania

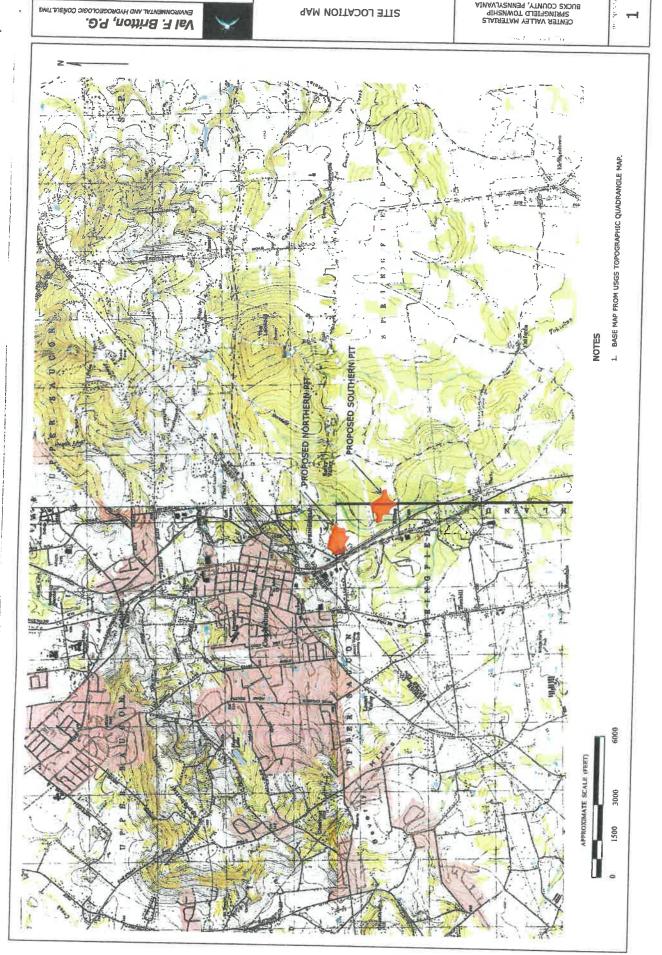
	Flow In (ft /day)	Flow Out (ft /day)
Source/Sinks		
Drains (Creeks)	0.00	20000 / 00
Recharge		292624,00
	292626,00	0.00
Total	292626,00	
% Difference	292020,00	292624.00
70 Difference		0.0010

FIGURES

SITE LOCATION MAP

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CENTER VALLEY MATERIALS
SPRINGFIELD TOWNSHIP
BUCKS COUNTY, PENNSYLVANIA



SITE LAYOUT PLAN

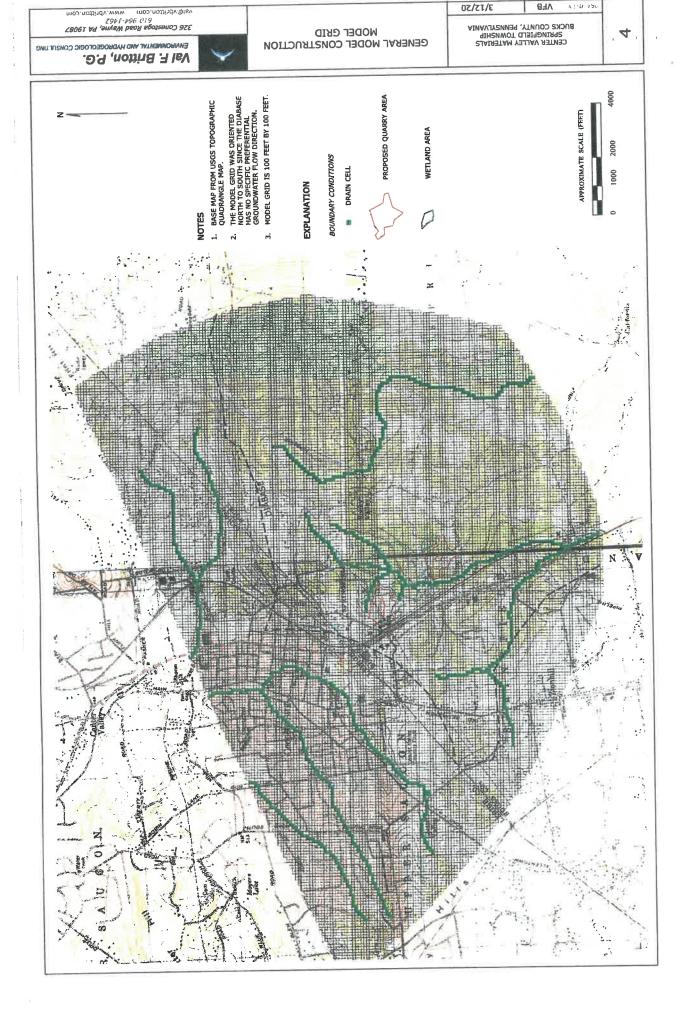
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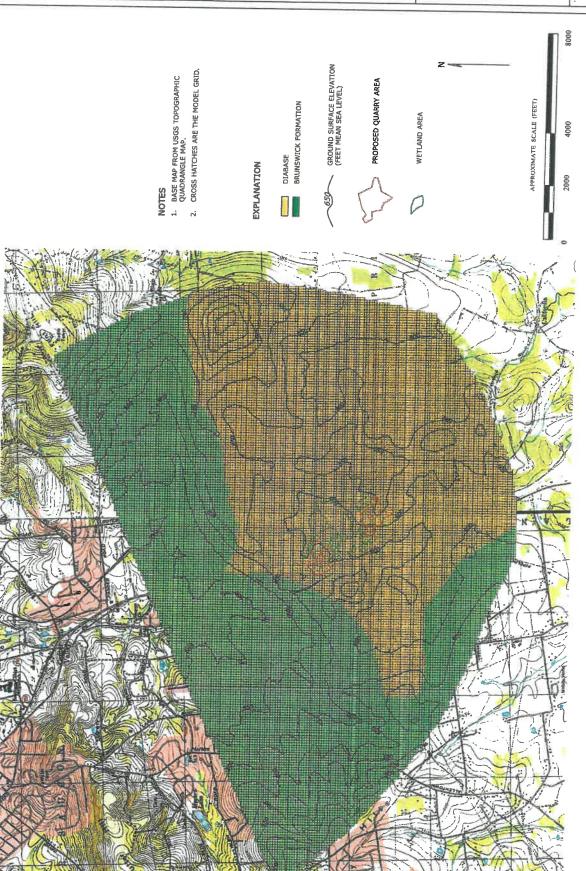
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DIABASE BRUNSWICK FORMATION BOTTOM OF MODEL

HYDROGEOLOGIC ZONES

EXPLANATION

VERTICAL EXAGGERATION 15 5X.

THE RECHARGE VALUE USED IN THE MODEL WAS 8.5
INCHES PER YEAR FOR THE BRUNSWICK FORMATION
AREA AND 2.0 INCHES PER YEAR FOR THE DIABASE.

GENERAL MODEL CONSTRUCTION LAYER CONFIGURATION AND HYDROGEOLOGICAL PARAMETERS

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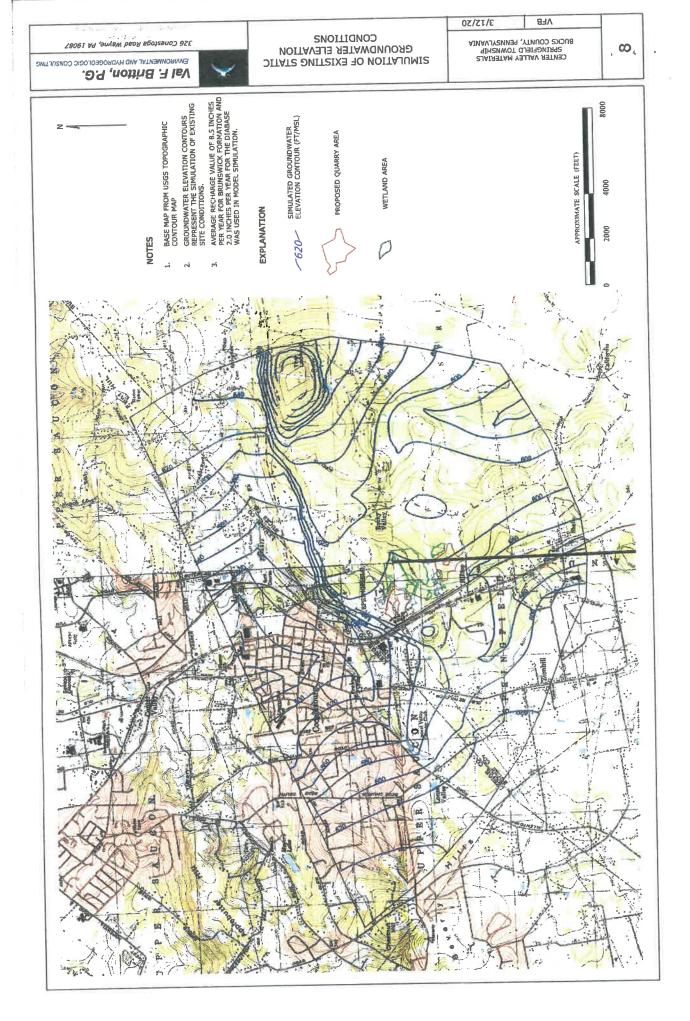
NORTHERN PIT

SOUTHERN PIT

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HYDRO-GEOLOGICAL PARAMETERS	OGICAL	PARAMETER	S
	k (fl/d)	k (ff/d) loy (ratio) kz(ratio)	kz(ratio)
Diabase (Layer 1)	0.1	-	-
Diabase (Layer 2)	0.01	-	0.5
Diabase (Layer 3)	0.001	-	0.5
Diabase (Layer 4)	0.0001	-	0.5
Brunswick (Layer 1)	0.328	-	-
Brunswick (Layer 2)	0.148	-	-
Brunswick (Layer 3)	0.148	-	-
Brunswick (Layer 4)	0.148	-	-
A service of	000	-	-

326 Conestoga Road Wayne, PA 19087 610, 964-1462 val@vhrtton.com www.vbritton.com 3/15/50 VF8 CENTER VALLEY MATERIALS
SPRINGFIELD TOWNSHIP
BUCKS COUNTY, PENNSYLVANIA MODEL CALIBRATION ENVIRONMENTAL AND HYDROGEOLOGIC CONSULTING Val F. Britton, P.G. AVERAGE RECHARGE VALUE OF 8.5 INCHES PER YEAR FOR BRUNSWICK FORMATION AND 2.0 INCHES PER YEAR FOR THE DIABASE WAS USED IN MODEL. BASE MAP FROM USGS TOPOGRAPHIC CONTOUR MAP GROUNDWATER ELEVATION CONTOURS REPRESENT THE SIMULATION OF EXISTING SITE CONDITIONS, 18 CALIBRATION RESIDUAL ERROR TABULATION IS PROVIDED ON TABLE 2. CHG MEAN ERROR: 5.02 FEET
ABSOLUTE MEAN ERROR: 8.07 FEET
ROOT MEAN SQUARE ERROR (RMS): 11.07 FEET
NORMALIZED RMS: 12.2% SIMULATED GROUNDWATER ELEVATION CONTOUR (FT/MSL) PROPOSED QUARRY AREA APPROXIMATE SCALE (FEET) WETLAND AREA CALIBRATION LOCATION CALIBRATION 器 Computed vs. Observed Values **→** 1000 **EXPLANATION** Served St -029-NOTES E S 570 B_₹ 100 9 550 8 25 8 H 049 83 S S S S 11 N



(400 FOOT BENCH ELEVATION) GROUNDWATER ELEVATION CONTOUR DEWATERING OF NORTHERN PIT NOITAJUMIS

CENTER VALLEY MATERIALS SPRINGFIELD TOWNSHIP BUCKS COUNTY, PENNSYLVANIA

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ENVIRONMENTAL AND HYDROGEOLOGIC CONSULTING Val F. Britton, P.G.



AVERAGE RECHARGE VALUE OF 8.5 INCHES PER YEAR FOR BRUNSWICK FORMATION AND 2.0 INCHES PER YEAR FOR THE DIABASE WAS USED IN MODEL SIMULATION. 8000 PURPLE SHADE INDICATES PIT DEWATERING AREA. GROUNDWATER ELEVATION CONTOURS BEPRESENT THE SIMULATION OF DEWATERING CONDITIONS OF THE NORTHERN PIT TO BENCH ELEVATION OF QUARRY (NORTHERN PIT) DEWATERING RATE BASED ON MODEL SIMULATION IS APPROXIMATELY 21 GPM. BASE MAP FROM USGS TOPOGRAPHIC CONTOUR MAP SIMULATED GROUNDWATER ELEVATION CONTOUR (FT/MSL) APPROXIMATE SCALE (FEET) **EXPLANATION** NOTES H

326 Conestoga Road Wayne, PA 19087 610 964-1462 val@whitton.com viww.vbritton.com SIMULATION

DEWATERING OF SOUTHERN PIT
GROUNDWATER ELEVATION CONTOUR
(400 FOOT BENCH ELEVATION) 10 BUCKS COUNTY, PENNSYLVANIA SPRINGFIELD TOWNSHIP CENTER VALLEY MATERIALS ENVIRONMENTAL AND HYDROGEOLOGIC CONSULTING Val F. Britton, P.G. QUARRY (SOUTHERN PIT) DEWATERING
RATE BASED ON MODEL SIMULATION IS
APROXIMATELY 17 GPM.
PUIRPLE SHADE INDICATES PIT DEWATERING
AREA. SIMULATED GROUNDWATER ELEVATION CONTOUR (FT/MSL) GROUNDWATER ELEVATION CONTOURS RERECENT THE SIMULATION OF DEWATERING CONDITIONS OF THE SOUTHERN PIT TO BENCH ELEVATION OF 400 FEET. AVERAGE RECHARGE VALUE OF 8.5 INCHES PER YEAR FOR BRUNSWICK FORMATION AND 2.0 INCHES PER YEAR FOR THE DIABASE WAS USED IN MODEL SIMULATION. BASE MAP FROM USGS TOPOGRAPHIC CONTOUR MAP APPROXIMATE SCALE (FEET) NOTES

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DRAWDOWN CONTOURS REPRESENT THE SIMULATION OF DEWATERING CONDITIONS OF THE NORTHERN PIT TO BENCH ELEVATION OF 400 FEET,

AVERAGE RECHARGE VALUE OF 8.5 INCHES PER YEAR FOR BRUNSWICK FORMATION AND 2.0 INCHES PER YEAR FOR THE DIABASE WAS USED IN MODEL SIMULATION.

BASE MAP FROM PEMA 2018 AERIAL PHOTOGRAPH, INTERIOR DRAWDOWN LINES REMOVED FOR CLARITY.

SIMULATION IS OF BENCH AT 400 FEET MEAN SEA

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CENTER VALLEY MATERIALS
SPRINGFIELD TOWNSHIP
BUCKS COUNTY, PENUSYLVANIA

Val F. Britton, P.G. Environmental and hydrogeologic consulting



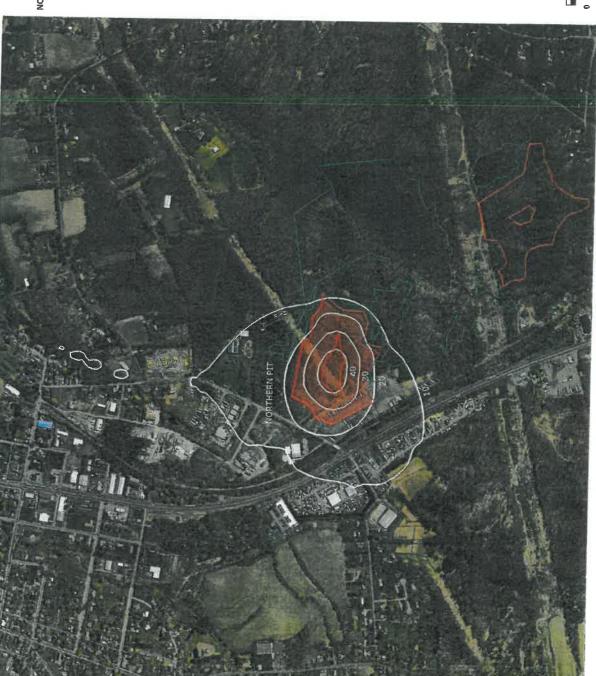
SIMULATION NORTHERN PIT DRAWDOWN (400 FOOT BENCH ELEVATION)

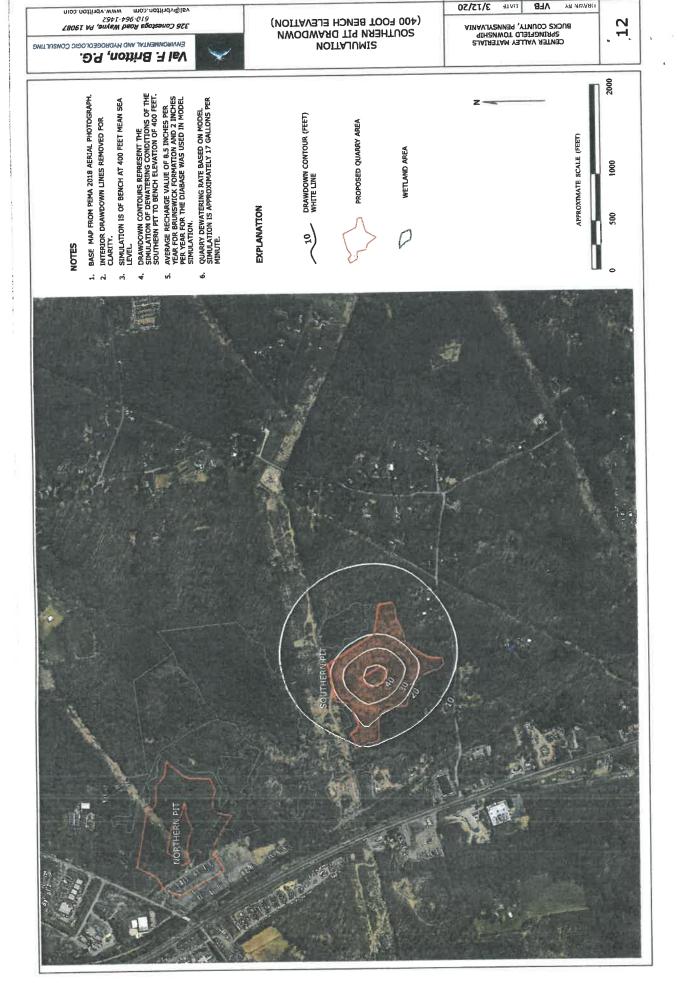
DRAWDOWN CONTOUR (FEET) WHITE LINE

07

PROPOSED QUARRY AREA

2000 APPROXIMATE SCALE (FEET) 1000 500





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YR AWARCI

Center Valley Materials
Springfield Township, Bucks County – Conditional Use Application

Attachment G

Plans

APPLICATION (G-7 QUARRY USE) CONDITIONAL USE

PREPARED FOR

VALLEY MATERIALS CENTER 1 INC. H&K GROUP.

SPRINGFIELD TOWNSHIP, BUCKS COUNTY, PENNSYLVANIA

MARCH 13, 2020









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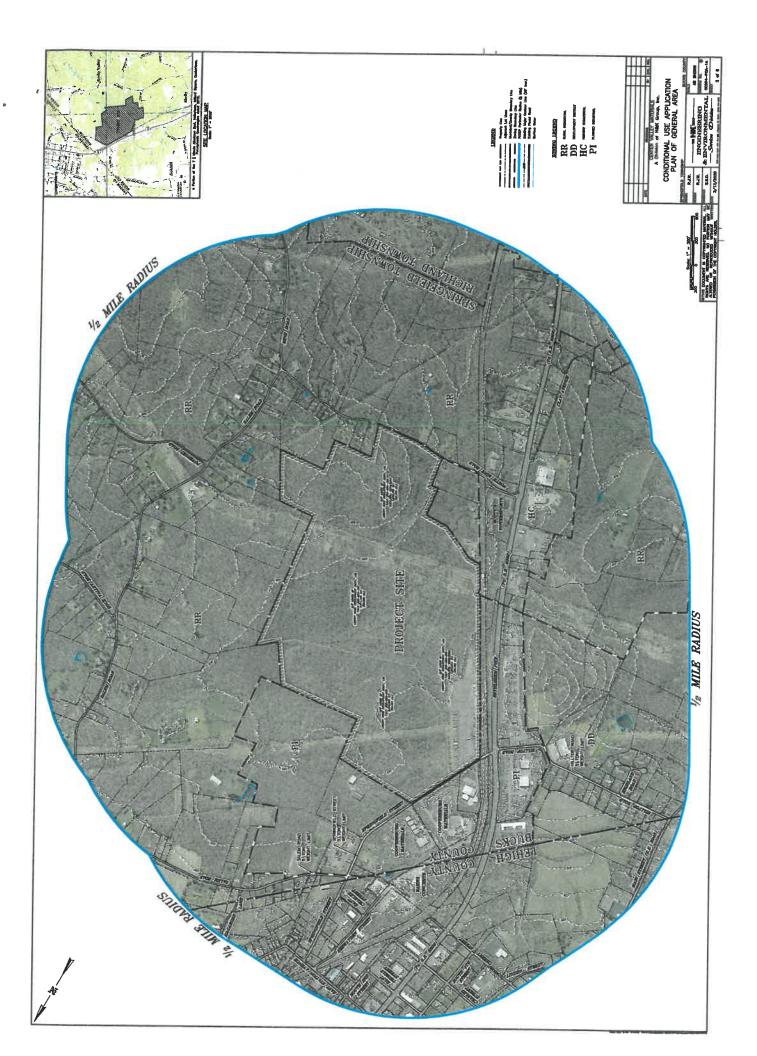
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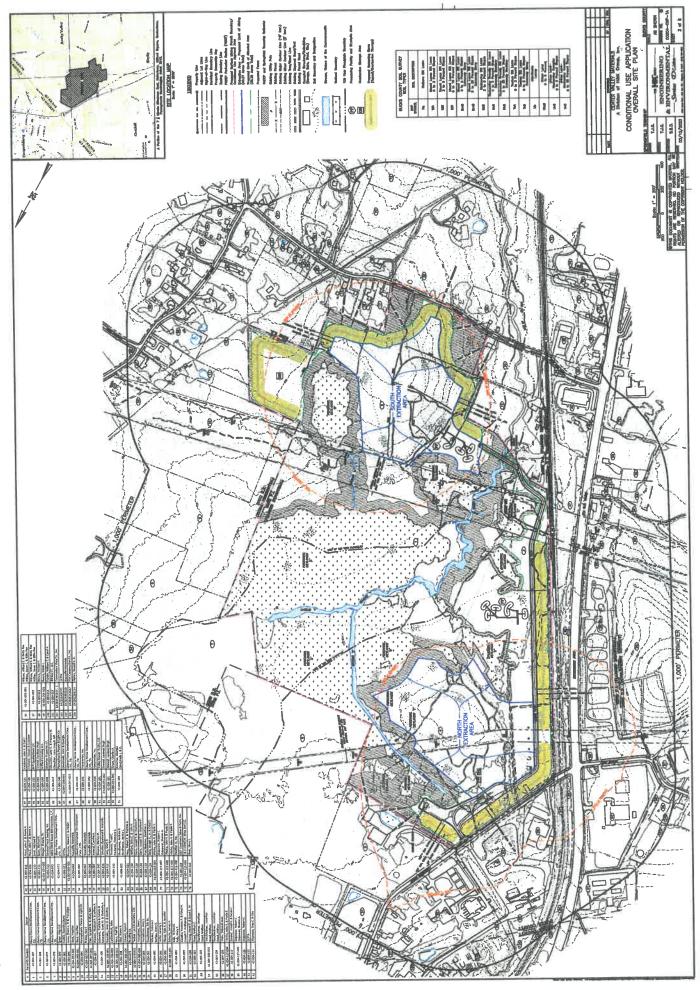
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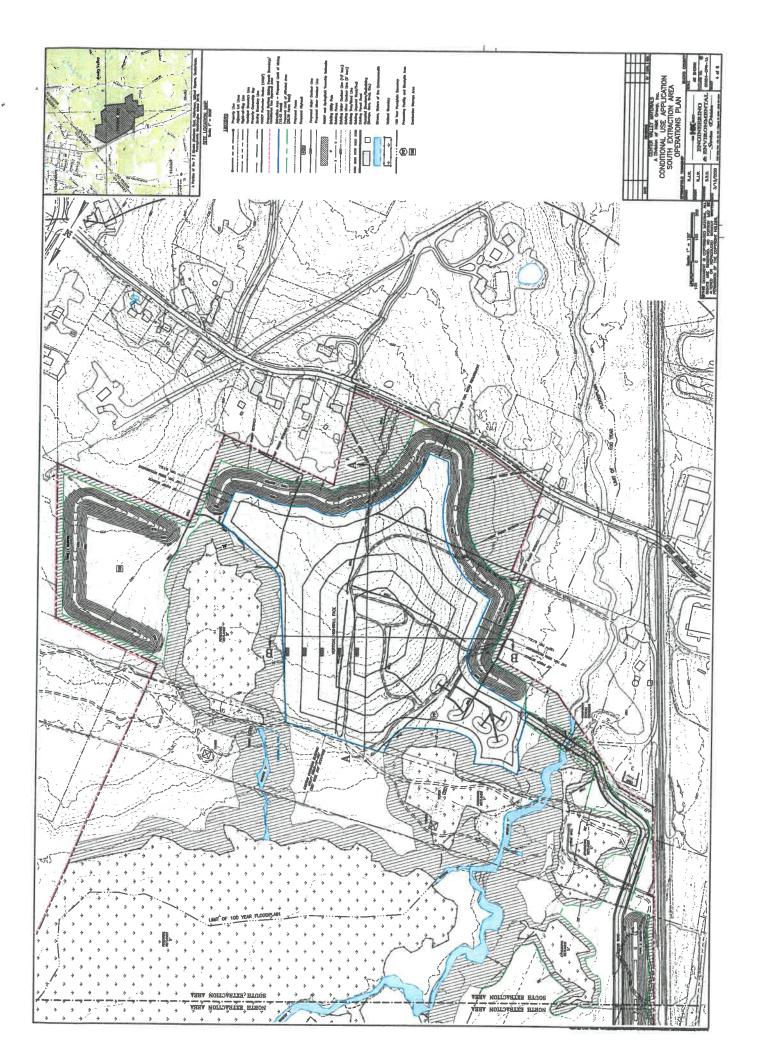
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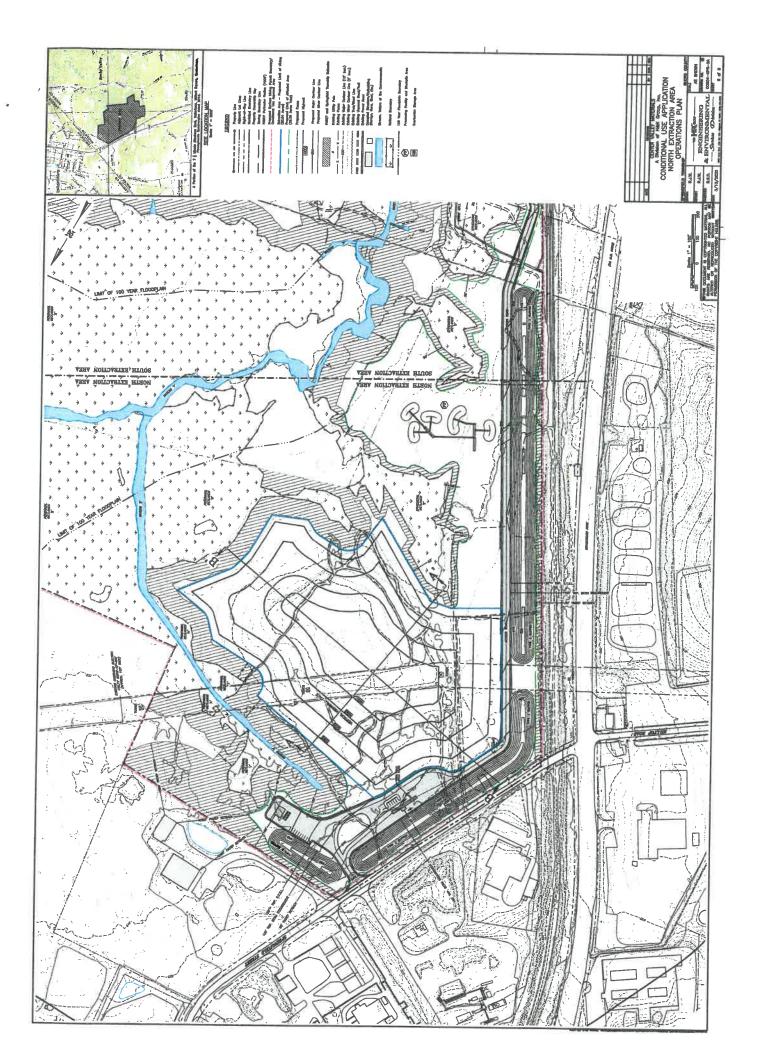


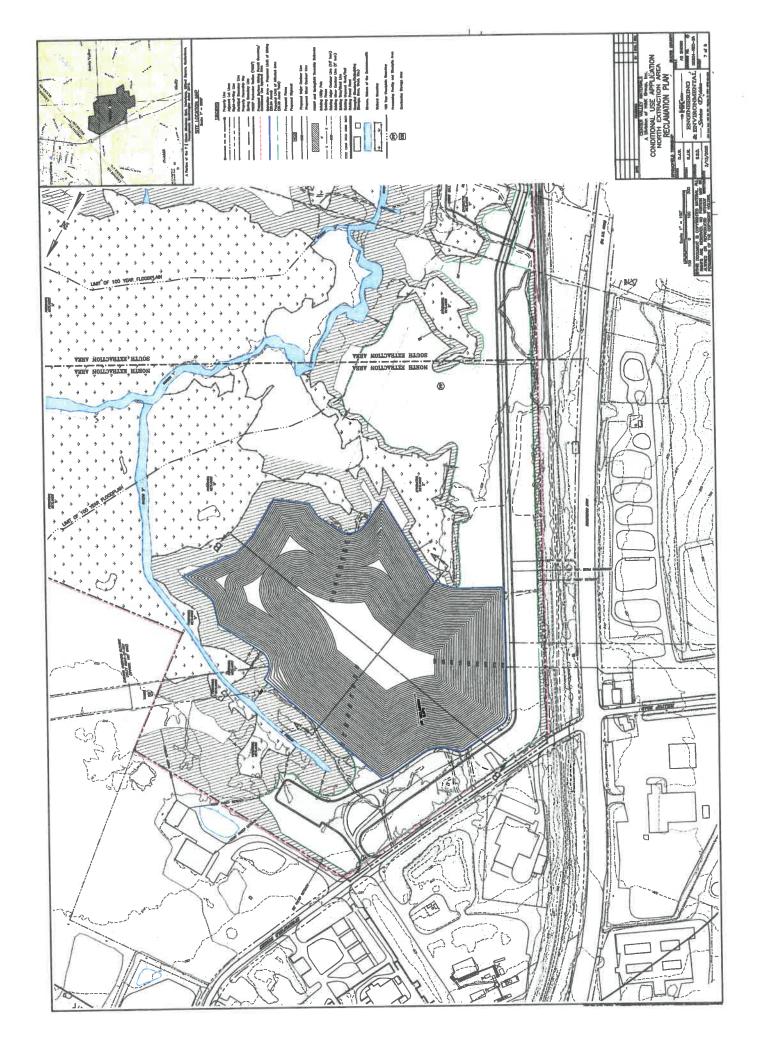


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